

Damage Observations and Issues Related to Building Performance

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Contents

- ◆ Damage Overview
- ◆ Findings from Surveys and Recommendations

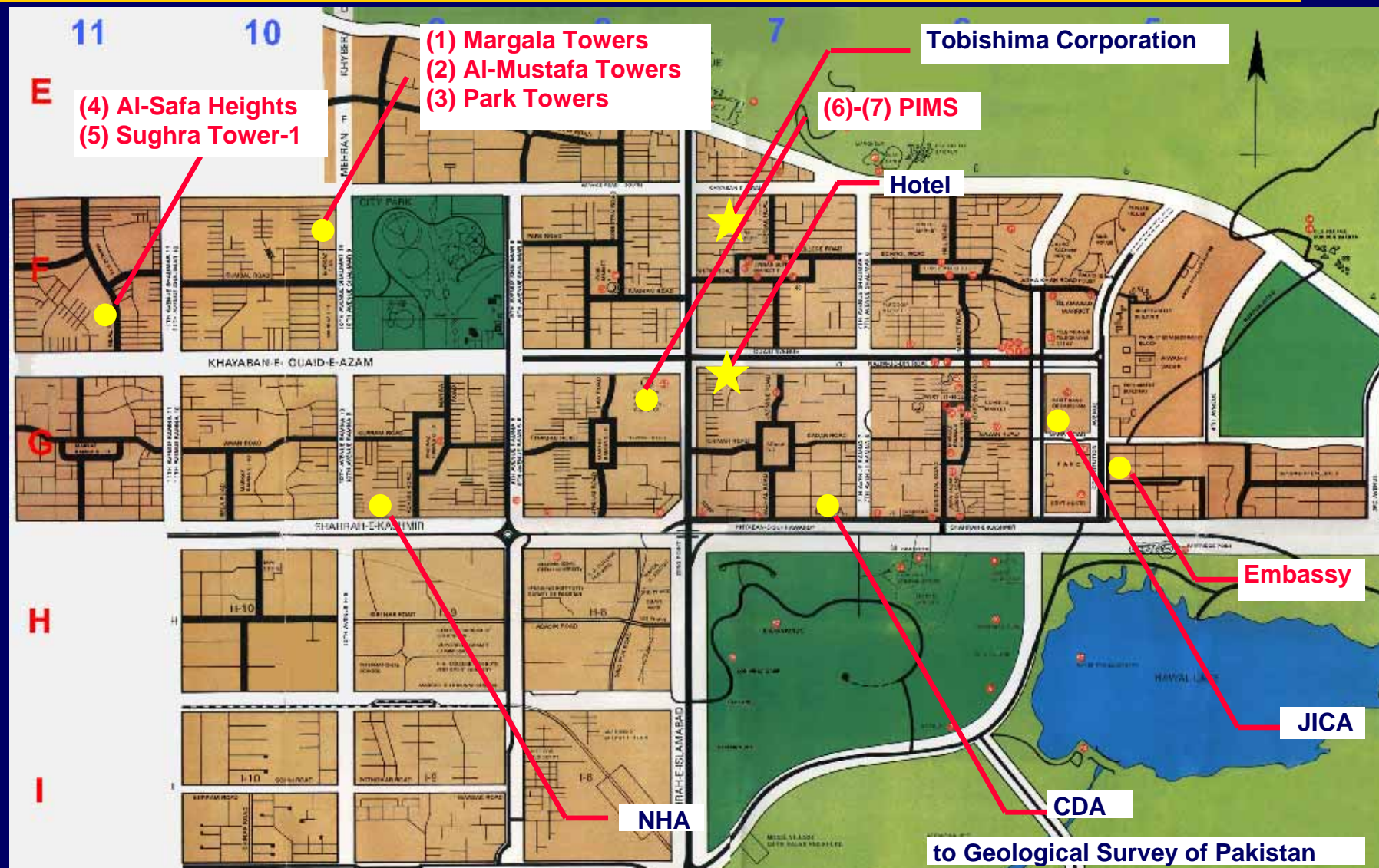
QUICK REPORT OF
DAMAGE INVESTIGATION ON BUILDINGS AND HOUSES
DUE TO OCTOBER 8, 2005 PAKISTAN EARTHQUAKE

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Surveyed Areas (1)



Surveyed Areas (2)



Islamabad Margala Towers



Islamabad Margala Towers



- ◆ Extensive Damage to URM Walls
- ◆ Damage to Expansion Joints
- ◆ However, No Major Damage to Structural Members



Islamabad Margala Towers

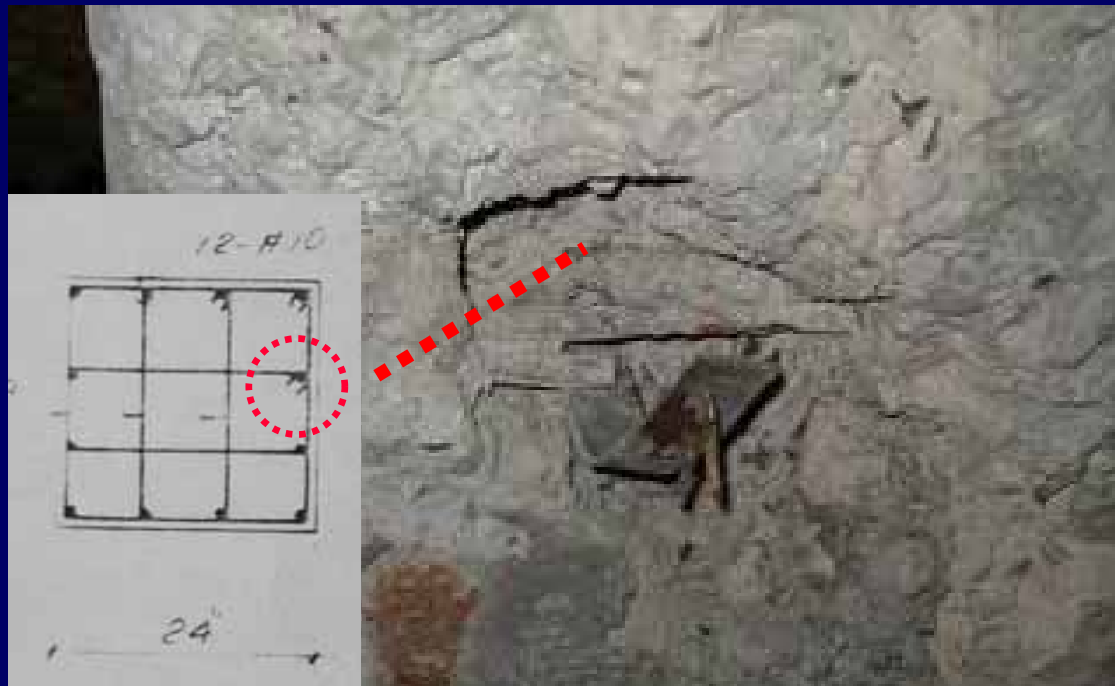
- ◆ Schmidt Hammer Tests in Building Nos. 2 - 4
 - Non-destructive test to estimate concrete strength from hardness of concrete surface.
 - It is often referred to estimate concrete strength especially when destructive tests are difficult to perform.

- ◆ Test results
 - 12 N/mm² (No. 2)
 - 17 N/mm² (No. 3)
 - 15 N/mm² (No. 4: collapsed)
 - ➔ Generally lower than requirements for Japanese high-rise buildings



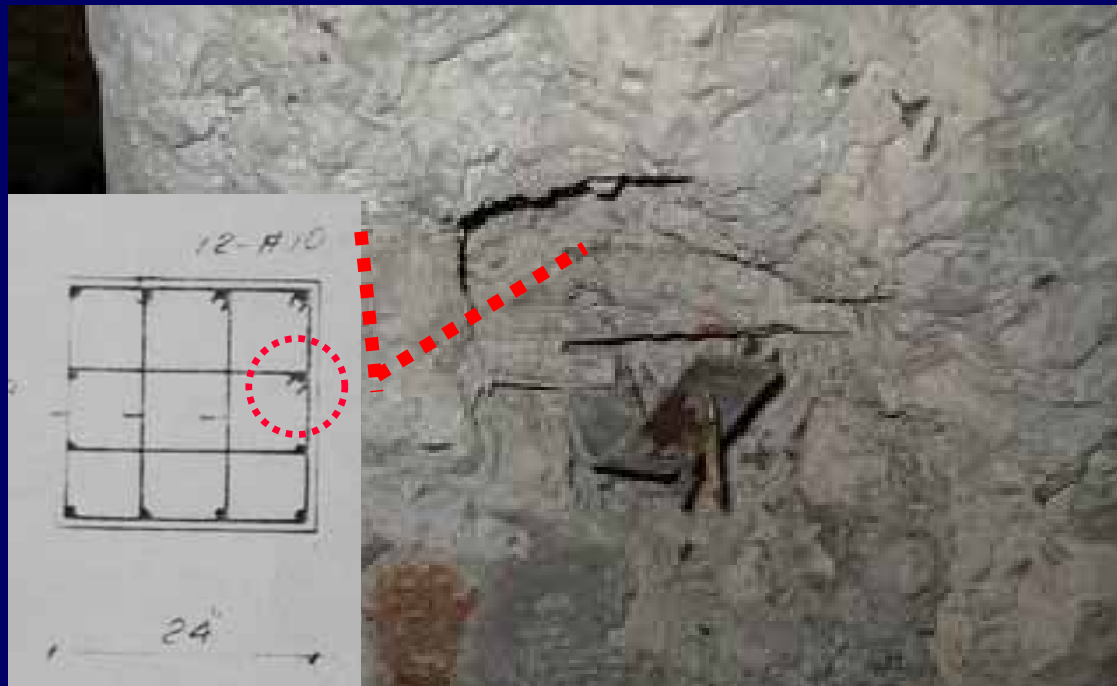
Islamabad Margala Towers

- ◆ Reinforcement Detections
 - 90-degree hooks although 135-degree specified in structural drawings



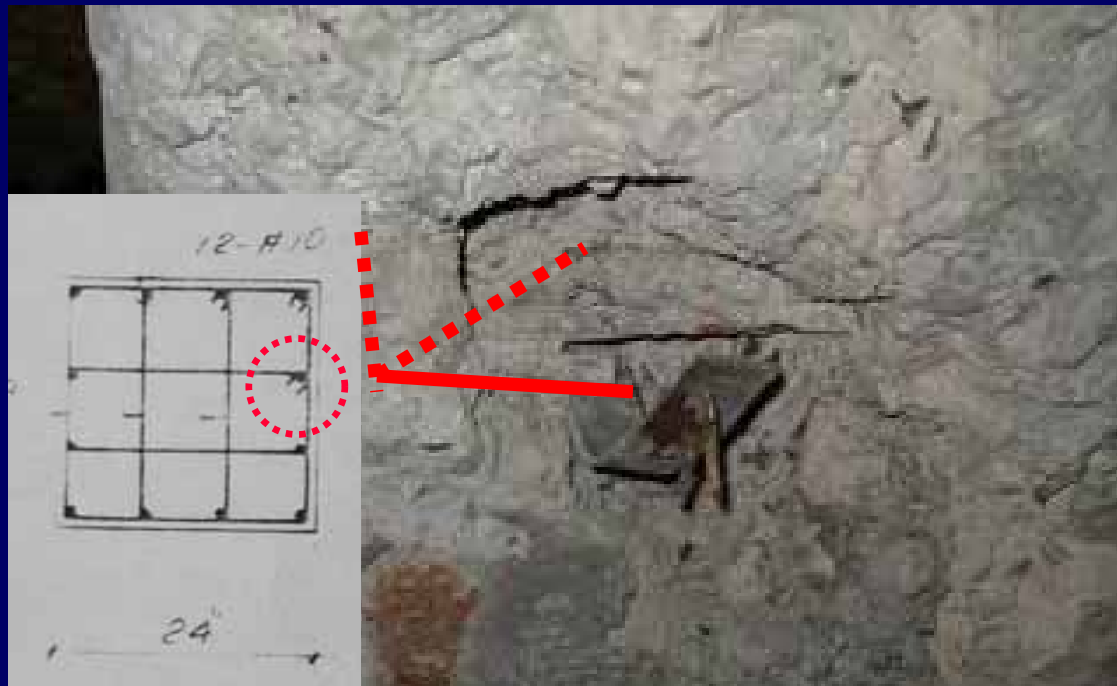
Islamabad Margala Towers

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Islamabad Margala Towers

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Islamabad Margala Towers

◆ Why the tower was collapsed?

→ Various Possible Reasons

- Low Quality of Concrete?
- Poor Reinforcement Detailing?
- Low Member Strength?
- Larger Building Weight?
- Soil Conditions?
- Soil Amplification?
- Long-period Ground Motions?
-

Islamabad Margala Towers

◆ Why the tower was collapsed?

→ Various Possible Reasons

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Possible Surveys

- ← Concrete core tests
- ← Rebar exposure
- ← Seismic Capacity
- ← Seismic Capacity
- ← Soil Profile
- ← Microtremor
- ← Response Spectra
-

Islamabad AI-Mustafa Towers

- ◆ Cracks in URM Walls
- ◆ No Visible Damage to Structure
- ◆ Schmidt Hammer
 - 28 N/mm²



Islamabad Park Towers

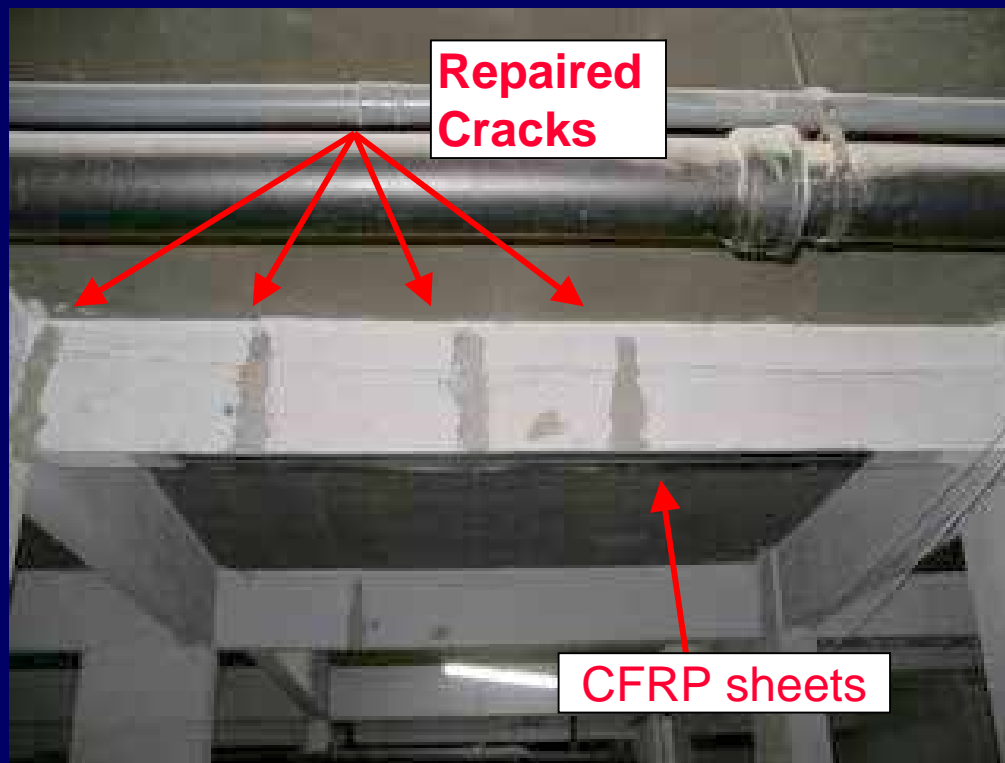
- ◆ Cracks in Beams of:
 - Basement
 - Open corridor in upper stories



Islamabad Park Towers

◆ Cracks in Beams of:

- Basement: Mid-span → Vertical Loads → Initiated before EQ
- Open corridor in upper stories EQ



Islamabad Park Towers

◆ Cracks in Beams of:

- Basement: Mid-span
- Open corridor in upper stories: Beam-ends → Caused by EQ



West Building



East Building



EQ

Islamabad Park Towers

◆ Cracks in Beams of:

- Basement: Mid-span
- Open corridor in upper stories: Beam-ends → Caused by

EQ



- No major difference in crack patterns between two buildings
- Two buildings behaved in the same manner
- Quantitative damage surveys are essential

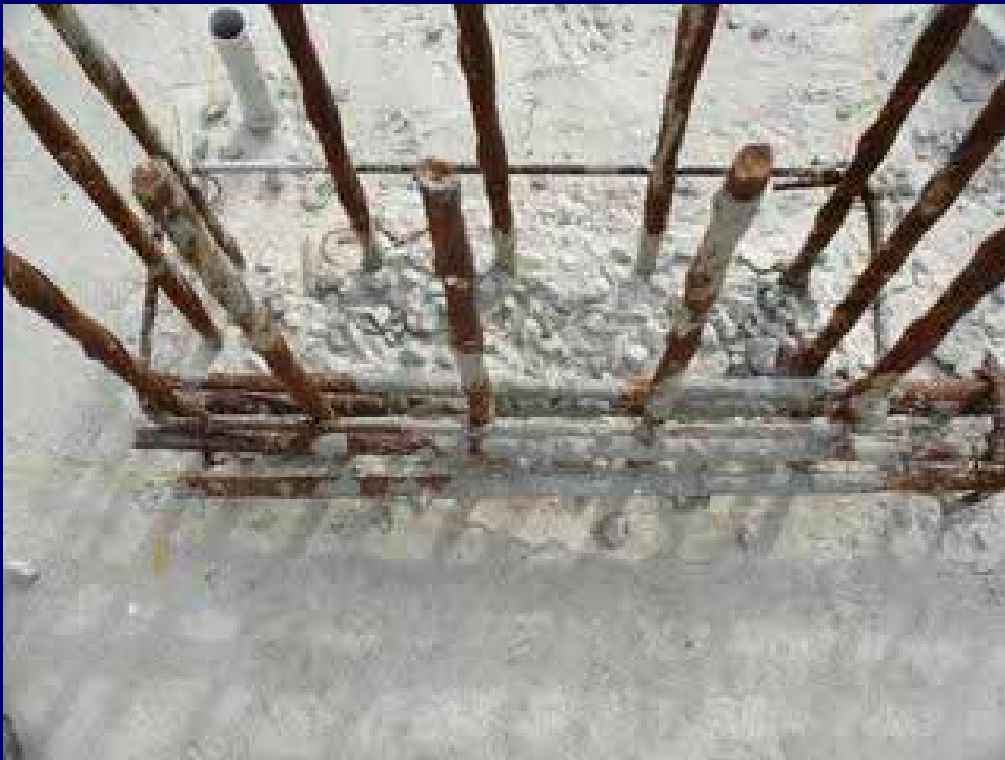
Islamabad Park Towers

- ◆ Pounding at Expansion Joints
 - Wider gaps between adjacent buildings are recommended



Islamabad Other Issues

- ◆ Poor Detailing of Rebars
 - Beam rebars out of column reinforcement cage
 - 90-degree hooks of shear reinforcement



Battal Primary School

- ◆ 2-story School Designed to Japanese Building Code
 - Flexural cracks at the top of a column
 - Extensive shear cracks in an URM wall



Battal Primary School

- ◆ Flexural Cracks Exposing Reinforcing Bars
 - Shear reinforcement: D10@10cm with 135-degree hooks
 - No buckling of main bars observed



Battal Primary School

- ◆ Evidence of Strong Shaking but Survived the EQ
- ◆ Schmidt Hammer Tests: 21 N/mm²
- ◆ Seismic Capacity Evaluation: $C_B=0.48$
(C_B = Strength / Building Weight : Base Shear Coef.)



Battal Jamia Mosque

- ◆ Columns Seriously Damaged
- ◆ URM Walls Had Extensive Shear Cracks
- ◆ Seismic Capacity Evaluation: $C_B=0.14$
($C_B = \text{Strength} / \text{Building Weight} : \text{Base Shear Coef.}$)



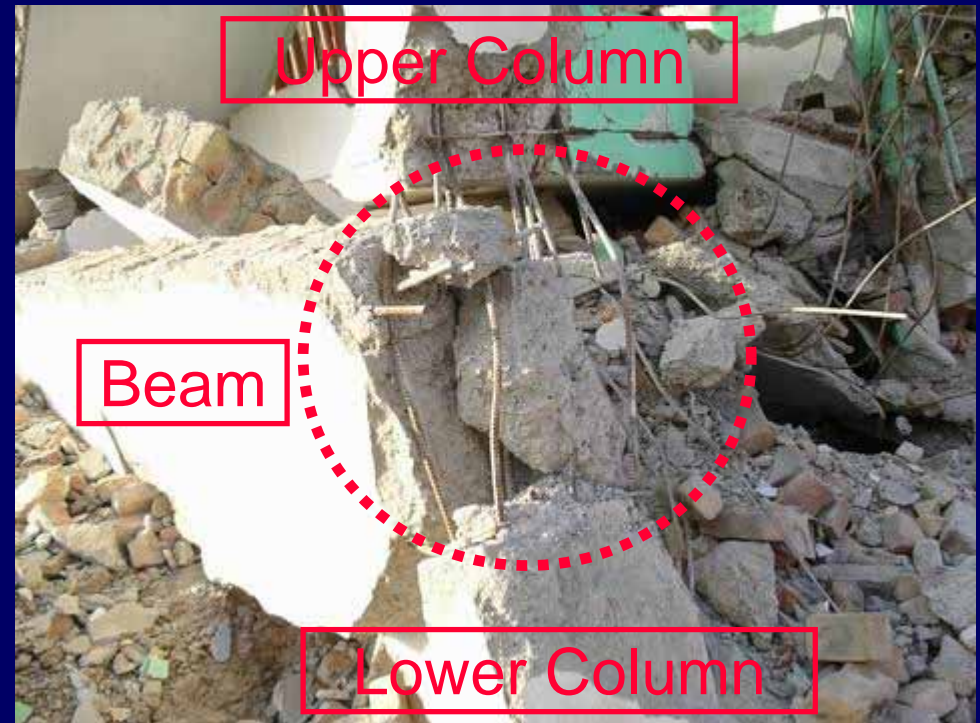
Abottabad

- ◆ Poor Shear Reinforcement in Members
- ◆ Poor Reinforcement Detailing in Beams



Abottabad

- ◆ Poor Shear Reinforcement in Members
 - Small diameter (6mm to 8mm) and wide space (30cm)
 - 90-degree hooks
 - No shear reinforcement in beam-column joints



Abottabad

- ◆ Poor Reinforcement Detailing in Beams
 - Anchored straightly into beam-column joints
 - No bent into core concrete
 - ➔ Beam-column connection failure
 - ➔ Pin-connected frames rather than moment resisting frames



Abottabad

◆ Poorly Repaired Damaged Buildings



Abottabad

◆ Poorly Repaired Damaged Buildings



Abottabad

- ◆ Poorly Repaired Damaged Buildings
 - ➔ Technical guidelines for post-earthquake rehabilitation are most needed



Abottabad

- ◆ Quality of Reinforcing Bars
 - Brittle failure without necking at fractured section
 - Also observed in other areas



Muzaffarabad

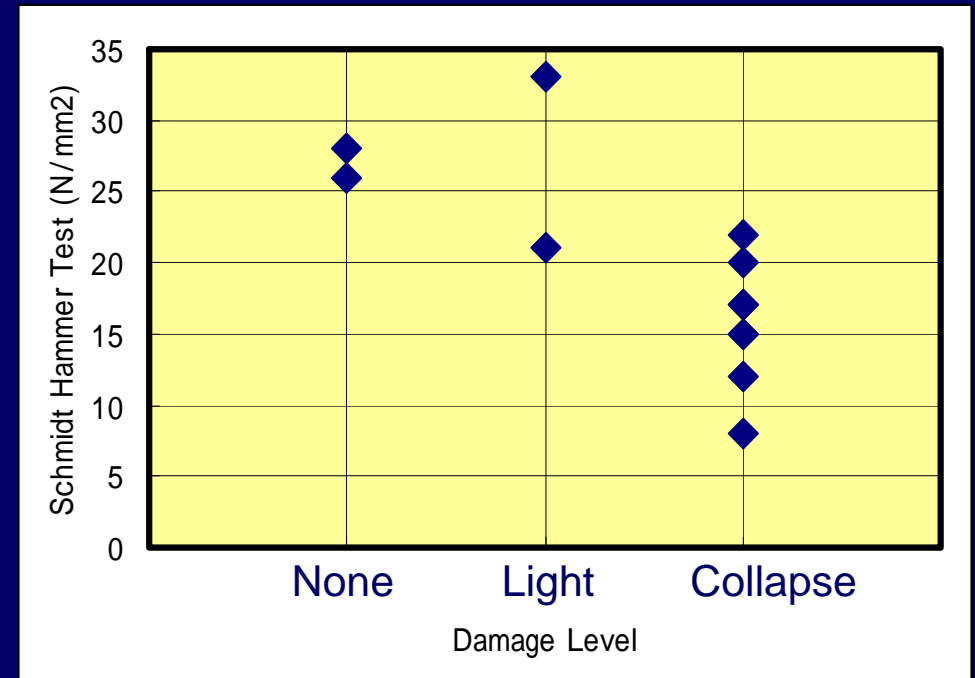
Muzaffarabad

- ◆ Vulnerable Buildings in Use
- ➔ Quick inspection of damaged buildings needed



Summary of Investigations (1/5)

- ◆ Concrete Quality
 - No ready-mixed concrete
 - Honeycombs
 - Coarse/fine aggregate
 - Damage vs. strength
- ◆ Reinforcement Quality
 - Brittle failure



Summary of Investigations (2/5)

- ◆ Properly anchored beam rebar in joints to avoid
 - Easy pulled-out failure
 - Pin-connected frames (→ moment resisting frames)
- ◆ Shear Reinforcement and Concrete Confinement
 - Too small and widely spaced shear reinforcement
 - 90-degree hooks (→ 135-degree hooks)for poor ductility and axial load carrying capacity



Summary of Investigations (3/5)

- ◆ Nonstructural Damage to URM (Unreinforced Masonry) Walls
 - Falling debris: hazardous to occupants
 - Lateral resistance and stiffness from RC and/or RM Walls
- ◆ Beam-Column Joints
 - No shear reinforcement in practice
 - Joint failure with buckling of rebars



Summary of Investigations (4/5)

- ◆ Pounding
 - Gaps large enough to avoid pounding
- ◆ Site Effects
 - Site effects and resulting amplified ground motions
 - Microtremor measurements
- ◆ Strong Motion Observations
 - Observed damage vs. input ground motions
 - Design EQ loads determination

Summary of Investigations (5/5)

- ◆ Quantitative Post-Earthquake Inspection
 - Safety evaluation of damaged buildings to aftershocks
 - Quick inspection system, procedures, and inspectors
- ◆ Rehabilitation Strategies and Techniques
 - Damaged sections not fully recovered to the original
 - Buckled rebars still in use
 - Tilted buildings (obviously hazardous) not re-centered
 - ➔ Technical guides to properly repair/strengthen damaged buildings